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1 Evidence of the impact of urbanization on the hydrological regime of a medium-sized periurban catchment in France Braud, I., Breil, P., Thollet, F., Lagouy, M, Branger, F. Irstea, UR HHLY, Hydrology- Hydraulics, CP 220, 3bis Quai Chauveau, F-69336 Lyon, France Jacqueminet, C., Kermadi, S., Michel, K. Université de Lyon, UMR CNRS EVS, 18 Rue Chevreul, 69363 Lyon Cedex 7, France Published in Journal of Hydrology, 485, 5-23, DOI: 10.1016/j.jhydrol.2012.04.049. 22Corresponding author: Isabelle BRAUD, Irstea, UR HHLY, CP 220, 3bis Quai Chauveau, F-2369336 Lyon, France. Isabelle.braud@irstea.fr 30 Journal of Hydrology, Special issue about the Hydrology of periurban catchments, 31DOI: 10.1016/j.jhydrol.2012.04.049

1Abstract

2In this paper we explore several indicators to evidence the impact of land use change, and 3particularly of urbanization/artificialization on discharge series of periurban catchments. A 4first set of indicators is derived from the literature and describes the monthly and annual 5hydrological regime, low flows and high flows, and flow components. Statistical tests are also 6applied to assess the existence of trends/ruptures on the longest time series. In addition, new 7indicators, especially built to show the impact of Sewer Overflow Devices (SODs) and 9catchment, located close to Lyon city (France) where various discharge gauges with a variable 10time step are available on sub-catchments ranging from a few to 130 km² (some of them 11nested), with a large variety of land uses (forest, agricultural land, artificialized areas). In 12addition, discharge is also measured in a SOD and a combined sewer network so that the 13relevance of the new proposed indicators can be assessed. In the largest sub-catchments, the 14 results show a decrease of specific discharge from upstream to downstream corresponding to 15an increase of artificialized areas, except for high flows. When a SOD is present, the specific 16discharge is increased for frequencies larger than 50%, and the frequency of zero daily 17discharge is decreased. Waste water can be the only source of water in autumn month in a 4.1 18km² sub-catchment. Base flow is also decreased for the most urbanized catchments. Our 19results confirm the impact of SODs on the modification of the flood regime, with an increase 20of frequent floods, but a marginal impact on the largest floods, mainly governed by saturation 21of the rural parts of the catchments. The decomposition of the sewer discharge shows that, on 22an annual basis, infiltration in the sewer network accounts for 30% of the total discharge and 23runoff due to rainwater to about 40% (the remaining being composed of the waste water 24discharge). It can explain the decrease of base flow. Our analysis shows that, for periurban 25catchments, a long term monitoring of nested sub-catchments and infrastructures (SODs, 26sewer networks) with a small time step, is very valuable and provides data allowing a 27quantitative assessment of the impact of urbanization on the whole hydrological regime.

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29Keywords: Suburban/periurban rivers, hydrological regime, high flow, low flow, impact of 30urbanization

11. Introduction

2

3Projections of population growth states that about 60% of the worldwide population is 4expected to live in towns in 2030 (Paul and Meyer, 2001). Around big cities, periurban areas 5are often the most affected by the corresponding urbanization (e.g. Meija and Moglen., 2010). 6In this paper, the word periurban, used in Europe and Australia, will refer to catchments made 7of a mixture of natural or agricultural lands, and urbanized areas. In the US, the word 8suburban is most often used, but it refers generally to residential areas with houses and 9gardens. Urbanization increases imperviousness of previously natural or agricultural areas. 10Construction of built-up areas is generally associated with the building of artificial structures 11such as road networks, drinkable, rainwater or sewer networks. These changes have an impact 12on the water cycle and aquatic ecosystems, due to the increase and acceleration of surface 13runoff, decrease of groundwater recharge and a modification of natural water pathways due to 14the artificial networks (e.g. Bras and Perkins, 1975; Chocat et al., 2001; Booth et al., 2002; 15Randhir, 2003; Matteo et al., 2006; Marsalek et al., 2007). This can lead to flooding, pollution 16and erosion problems within periurban rivers. In some areas, equipped with Combined Sewer 17Systems (CSSs), Sewer Overflow Devices (SODs) are introduced. When the sewer network is 18overflowed, it delivers polluted water to the river. This can lead to incision and erosion 19problems, which perturb the ecological status of the rivers (e.g. Hatt et al., 2004; Walsh et al., 202005; Lafont et al., 2006). In addition, periurban catchments have a complex structure, made 21of a mixture of natural, agricultural and urbanized areas, and are evolving very quickly (e.g. 22Beighley et al., 2003; Radojevic et al., 2010; Jankowfsky, 2011).

23Studies of land use change impact on the hydrological regime generally analyse long term 24time series or long term simulations, and not only events. The objective is not to reproduce 25some historical events for sizing hydraulic works, but to get an appraisal of the change of the 26whole river regime. Appropriate criteria and indicators relevant to reveal the impact of land 27use change on this river regime must therefore be defined. In the following, the discussion 28will be restricted to indicators related to quantitative hydrology. Lots of other indicators have 29been defined to quantify the ecological status of receiving waters, based on chemistry, 30biology, etc (e.g. Walsh et al., 2005), in particular in Europe with the Water Framework 31Directive. But they are beyond the scope of this paper. In the remaining of the paper, the word 32indicator will be used in a broad sense and will include both hydrological variables and 33quantities derived from discharge data analysis.

1The literature dealing with the impact of climate change, land use/ land cover change on the 2water cycle and hydrological model evaluation was reviewed. The objective was to identify 3the hydrological indicators used by their authors to quantify the impact on discharge data. A 4review of climate change studies is provided by Praskievicz and Chang (2009). Examples of 5land use change studies, mainly related to the impact of deforestation/aforestation on the 6water cycle can be found in Ott and Uhlenbrook (2004), Wang et al. (2007), Archer (2007) 7and Bathurst et al. (2011). These studies are either based on model results or on data analysis 8(Table 1). A large number of the experimental studies are based on paired catchments, where 9natural and preserved catchments are compared to disturbed catchments (Table 1). Vazquez et 10al. (2008) and Willems (2009) introduced several criteria which provide information on the 11performance of a model for several ranges of discharges. These indicators include the 12components of stream flow: base flow, inter flow and quick flow, as well a Peak Over 13Threshold (POT) analysis (Stedinger et al., 1992) both for low flows and high flows. From 14this literature review we have extracted 5 classes of indicators used to examine the impact of 15land use/land cover change on discharge time series. They are summarized in Table 1, which 16also presents the corresponding references.

17

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19The first class of indicators in Table 1 is related to the hydrological regime where the 20indicators are the mean annual runoff, its seasonal components, discharge quantiles and flow 21duration curves.

22The second class of indicators is related to high flows: value and/or date of the annual 23maximum discharge, peak over threshold (POT) analysis which studies peak flow. The QdF 24– Discharge – duration – frequency analysis (Galéa and Prudhomme, 1994, 1997; Javelle et 25al., 1999), an extension of POT analysis to different characteristic durations and not only to 26the instantaneous maximum, is also used to study high flows.

27Low flow indicators form the third class of indicators. They include value and date of 28minimum annual discharge, frequency of zero discharge, base flow index defined as the ratio 29between annual base flow and total annual flow, POT analysis for low flow (Willems, 2009).

30The fourth class of indicators is based on hydrograph analysis. It includes the study of event 31characteristics (runoff coefficient, rising and falling limbs of hydrographs), the quantification 32of flow components based on the separation into base flow, interflow and quick flow (Blume 33et al., 2007; Willems, 2009). Archer (2007) and Archer et al. (2010) introduce more 34sophisticated indicators based on the analysis of rising and falling limbs of discharge series,

lor their rate of change. These indicators are the number and duration of exceedance of 2multiple of the annual discharge. These indicators are relevant to identify quick disturbances 3in hydrographs.

4Finally the fifth class of indicators is based on statistical analyses of discharge time series, 5mainly relevant when long time series are available. This includes the results of statistical 6tests, comparing if differences between various periods are significant (e.g. Tong, 1990; 7Wang et al.,2007; Radojevic et al., 2010), the results of trend analysis (e.g. Claessens et al., 82006). The results of regression analysis or neural network models, proposed by some authors 9to relate hydrological characteristics to climate or landscape variables such as imperviousness 10(Tetzlaff et al., 2005; Dow, 2007; Wang et al., 2007; Arrigoni et al., 2010) can also be 11included in this fifth class.

12

13In urbanized areas, as can be seen from Table 1, the analysis is often restricted to mean annual 14runoff (Beighley et al., 2003; Claessens et al., 2006); high flow with indicators such as annual 15peak discharge (Beighley et al., 2003; Burns et al., 2005), QdF analysis (Radojevic et al., 162010) and/or to the study of selected events (Ott and Uhlenbrook, 2004; Chormanski et al., 172008; Meierdiercks et al., 2010). But this may not be sufficient for all applications, in 18particular when addressing the quality of receiving waters. For ecological problems, the 19whole hydrological regime, and in particular low flows are also important. Walsh et al. (2005) 20suggest that a good target for the rehabilitation of rivers affected by urbanization would be to 21go back to "near natural" surface flow conditions. This requires the characterization of the 23hydrological regime to compare pre and post river flow management with an emphasis to 24stream ecology are proposed by Richter et al. (1996, 1997, 2003). It includes timing of annual 25extreme water conditions; frequency and duration of high/low pulses; rate/frequency of water 26condition changes. Jacobson (2011) also provides a review of the impact of imperviousness in 27urban catchments.

28Response times of impervious areas are much shorter than those of natural areas. For small 29periurban catchments, with response time of less than one day, indicators based on daily 30discharge may not be fully relevant. In addition, given the very short functioning time of 31SODs, specific analyses are required to fully evidence the impact of the urban areas on the 32river flow, especially for small catchments (a few km²). Therefore the analysis of indicators 33based on smaller time steps should be contemplated. Some authors also suggest that 34periurbanization mainly affect frequent floods (e.g. Radojevic et al., 2010) like observed

1when studying the impact of forest or deforestation on floods (Bathurst et al., 2011). 2Therefore indicators based only on peak discharge or extreme events may not be relevant for 3assessing the impact of urbanization on the water regime, and the whole range of floods must 4be investigated. Another specificity of periurban catchments is the impact of sewer networks, 5installed to manage both waste water and rainwater. In general those networks are not 6watertight and infiltration of clear water in the sewer system is very common (e.g. Breil et al, 71993; Berthier et al., 2004; Rodriguez et al., 2008). This may have an impact on the low flow 8within the rivers (e.g. Gufstafsson et al., 1997) and should also be considered.

9In order to characterize the impact of urbanization on periurban rivers discharge, paired 10catchments studies, comparing a natural and an urbanized catchments would be the best way 11to get the answer. When trying to assess land use change impacts on discharge, it is also 12sometimes difficult to distinguish between climate variability and land use impact (e.g. 13Ashagrie et al., 2006). Sub-periods comparison for instance may be affected by climate 14variability which can lower the significance of the results. Some authors have proposed 15methods to filter climate variability and highlight the impact of land use by removing climate 16variability using regressions from a reference period, in general considered as undisturbed 17(e.g. Dow, 2007; Wang et al., 2007; Arrigoni et al., 2010). However, those methods may be of 18limited use when recorded data have started as the same time as urbanization (i.e. in the 191970s, as it is the case in France). And it is not obvious to find "natural" catchments other 20things being equal. Therefore, there is a need to develop specific methods which cope with 21 existing data series and try to separate the impact of land use change/ urbanization/ 22management structures from the natural variability of discharge time series. This direction is 23 explored in this paper for small to medium-sized periurban catchments, that range from some 24hectares to some tenth/hundreds of squared kilometres. This analysis is complementary to the 25set of indicators identified in the previous literature review.

26

27The objectives of the paper can therefore be stated as follows: are we able to define indicators 28revealing and possibly quantifying the impact of land use change and in particular 29artificialization on the hydrological regime of small to medium size periurban catchments? 30The study is conducted in two steps. First some indicators characterizing the discharge time 31series, derived from the above cited literature, are assessed. Then, given their identified 32limitations, essentially in taking into account the effect of sewer systems and SODs, specific 33approaches, adapted to periurban catchments are introduced and discussed. The methods are 34applied to the Yzeron catchment, located close to the city of Lyon, France where quite long

1 time series of both rainfall and discharge are available for nested sub-catchments 2 encompassing various ranges of urbanization and sizes. The results are discussed in order to 3 define the advantages/limitations of the retained indicators.

4

52. Case study

62.1. Context of the study and presentation of the catchment

7The Yzeron catchment (150 km²) is located to the south-west of Lyon city (Figure 1). It forms 8part of the Observatoire de Terrain en Hydrologie Urbaine (OTHU, 2011) long term 9observatory. It is representative of French periurban areas and is characterized by a marked 10topography (Figure 1). The outlet reaches the Rhône river at the elevation of 162 m and the 11highest points culminates at 917 m above see level. The slope map calculated from a 25 m 12resolution Digital Elevation Model (DEM), derived from IGN BDTopo® shows that more 13than 50% of the catchment has slopes larger than 10% (Gnouma, 2006). The geology is 14contrasted with crystalline formations (granite, gneiss) in the western part of the catchment 15and more alluvial and glacier formations in the eastern part. This led to a complex soil types 16map with 22 cartographic units identified in the soil map (SIRA, 2011). The land use is 17heterogeneous. The upstream and western part of the basin is limited by a range of hills 18covered with forests. The intermediate part is mainly covered with grassland and cultivated 19lands, mixed with urban nucleus. Thin green corridors remain along rivers, covered with 20deciduous trees. The downstream part is mainly covered with densely urbanized areas (Figure 212c).

22A fast progression of urbanization is observed since the eighties (Cottet, 2005; Gnouma, 232006, Radojevic et al., 2010). This evolution generally develops in the form of small urban 24centres that expand from old villages, along road networks, following topographic constraints 25that are imposed by the river networks. A recent analysis was conducted to quantify the 26increase of urbanization in this catchment (Jacqueminet et al., 2011; Kermadi et al., submitted 27for publication). A manual digitalization of aerial photographs allowed the identification of 28artificialized surfaces including the urban parcels, parkings, industries and their green fraction 29(gardens, parks) (Figure 3). The percentage of artificialized surfaces was 22% of the 30catchment area in 1970, 33% in 1990 and 36% in 2008 (Figure 2), showing a slight decrease 31in the rate of urbanization between 1990 and 2008, as compared to the period 1970-1990. A 32similar analysis based on SPOT images analysis allowed the quantification of impervious 33areas which were found to cover 15% of the catchment area in 1990, 18% in 1999 and 23% in

12008. The difference between both image treatments can be seen by comparing Figure 1 2which shows the impervious areas and Figure 2c, which shows the artificialized areas for year 32008.

4The area is prone to sharp Mediterranean-type flood events due to its steep topography in the 5upstream part and limited soil water storage capacity overall. The response time of the 6catchment is about 12 hours at 130 km², causing sometimes flooding in the downstream town 7of Oullins. An increase in the frequency of flooding has been observed in the recent years 8(Radojevic et al., 2010). The water responsible from these flooding mainly comes from the 9rural part of the catchment, but its effect can be enhanced by the fast contribution of urbanized 10zones. The water coming from combined sewer overflow devices is rich of sediments and 11pollutions, causing quality problems in the rivers, especially during summer storms, where 12most of the water reaching the river comes from urbanized areas via SODs (Lafont et al., 132006). Increased erosion of the river banks has also been evidenced with impact on the 14ecosystems (Schmitt et al., 2008).

15The Yzeron catchment was recently studied within the framework of the AVuPUR (Assessing 16the Vulnerability of Peri-Urban Rivers) project (Braud et al., 2010). The aim of the project 17was to increase the knowledge of the functioning and behaviour of periurban catchment 18hydrology and to propose modelling tools, adapted to those catchments. The study presented 19in this paper is a contribution to this project. The indicators discussed here are also relevant 20for model evaluation.

21

222.2. Available data

23<u>Rainfall and climate data</u>: A network of rain gauges was set up over the Grand Lyon area in 241985. It includes 28 rain gauges, 4 of which are located within the Yzeron catchment, but 25mainly in the eastern part of the catchment (Figure 1). The rainfall is recorded continuously 26with a 6 minutes time step and all the data set is available until 2010. In order to better 27document the research catchment and the mountainous area, Cemagref has also installed 28complementary rain gauges with a variable time step. The oldest ones were installed in 1997 29(Pollionnay and Grézieu) and the network is continuously upgraded since then. The daily data 30(1921-2009) from the synoptic Bron station, located about 20 km east of the catchment, were 31also used.

32The SAFRAN reanalysis data set (Quintana-Segui et al., 2008; Vidal et al., 2010), which 33provides the climate variables over a 8 x 8 km² grid is also available for the grid points

1 covering the Yzeron catchment over the 1970-2010 period with an hourly time step. This data 2set is the only one available to document the climate (air temperature and humidity, wind 3speed, long and short wave radiation). The SAFRAN data were used to compute the reference 4evapotranspiration, PET, following the FAO (1998) method.

5

6Stream flow data: Two gauges from the national HYDRO data base, maintained by DREAL 7Rhône-Alpes are available on the catchment: the Craponne station (48 km²) since 1969, and 8 (Figure 1). These two stations are nested. The data 9are available with a variable time step. Three other stations from Cemagref are also available. 10They sample the Mercier (7 km²), Chaudanne upstream a sewer overflow device 11(Upstream SOD1) (2.19 km²), Chaudanne at La Léchère (2.9-4.1 km²) experimental sub-12catchments (Figure 1). The last two stations are nested. The data are registered with a variable 13time step. In addition, in the Chaudanne catchment, data are collected at the outlet of the 14sewer overflow device (SOD1) since 2001. Discharge is also measured since 2001 in the 15combined sewer system (CSS) which is directed to the WWTP but sometimes overflows in 16the SOD1 (Figure 3). The main characteristics of the gauging stations are summarized in 17Table 2 and Figure 4 provides the land use encountered within the sub-catchments, using 18three main classes: wooded areas, agricultural land and artificialized areas. We must also 19mention the existence of additional SODs just upstream the gauging stations of La Léchère 20(SOD2) (Figure 3) and Craponne. The methodology presented in section 3.2 will be used to 21 assess the impact of these SODs on the discharge measured at the downstream gauging 22stations.

23The discharge gauging stations network is quite original due to the range of scales and variety 24of land uses which are sampled (Table 2 and Figure 4). The length of the series is also very 25valuable to evidence the impact of land use change on river discharge.

26

272.3. Summary of previous researches

28The Yzeron catchment is studied for about 20 years. In the following, we only summarize the 29studies directly related with hydrology and water fluxes. Radojevic (2002) and Radojevich et 30al. (2010) analysed the rainfall and stream flow data (Craponne and Taffignon stations) over 31the 1969-1978 and 1988-1997 decades. The rainfall series showed significant differences 32between both decades, with an increase of maximum daily rainfall in the 1988-1997 period. 33This analysis was updated using the climatic data presented above (Bron rainfall daily time

1series and SAFRAN daily climatic data) in order to detect possible changes in the 2climate/rainfall forcing (Kermadi et al., 2011; Braud, 2011). On the rainfall data, a significant 3increase (Mann Kendall test (Mann and Whitney, 1947) at the 5% level) of the annual 4maximum of 5, 10, 15-day cumulative rainfall was found for the Bron station in the 1920-52010 period, with a rupture detected in 1974 by the Pettitt test (1976). No significant 6trends/ruptures were found when the period was restricted to the 1970-2009 period. The 7analysis of the daily SAFRAN data on the 1970-2009 period showed a significant increase (at 8the 1% level) of the annual mean temperature of 0.52 °C every 10 years. But no significant 9trend in annual PET was found. On the other hand, the Pettitt test show a significant (at the 105% level) rupture of the annual PET in 2002, which will have to be confirmed in the future. 11From all the results obtained on the daily rainfall, temperature and PET data, we concluded 12that it was unlikely that change in the rainfall and climate could have affected the 13hydrological regime on the 1970-2010 study period. However, in the context of urban 14catchments with short response times, such an analysis should also be conducted with shorter 15time steps. It will be done when longer short time step rainfall series will be available (they 16are presently only 25 years long).

17In the study by Radojevic et al. (2010), the stream flow series showed an increase of frequent 18floods, whereas large floods were not so affected. The change in frequent floods was 19attributed to the impact of urbanization. Radojevic et al. (2010) also used a modelling 20approach to asses the possible impact of urbanization on the hydrological regime, through the 21impact on the discharge – duration – frequency (QdF) curves. But their analysis is restricted 22to two short periods of about 10 years, and more than 10 years additional data are now 23available. Gnouma (2006) analysed the data from the Craponne, Mercier and Chaudanne sub-24catchments, and tried to propose monthly water balance. The analysis was restricted to the 251997-2004 period.

26In the present study, the discharge data analysis will be extended to include the whole 27available period (until 2010) and all stream flow gauges will be considered. The aim is the 28assessment of indicators, relevant to evidence the impact of land use change on the discharge 29time series. These indicators include annual and monthly discharge, base flow index, peak 30over threshold (POT), QdF analysis and flow components deduced from hydrographs 31decomposition. The analysis provides elements for a better understanding of the catchment 32behaviour, both for its rural and urban components. The derived data and synthesis are also 33useful for the evaluation/validation of the models developed within the AVuPUR project.

23. Methodology

3In section 3.1, we first assess indicators taken from the five classes that were identified in the 4literature review, as our objective was to qualify the impact of land use change and 5urbanization on the whole hydrological regime: flow duration curves, low flows, high flows. 6The flow components derived from the hydrograph decomposition were also considered as 7urbanization is suspected to increase quick flow and decrease base flow. Finally, as one of the 8discharge series was long enough, statistical tests (trends/ruptures) were also applied to this 9time series.

10In section 3.2, we present methods specifically developed to evidence the impact of sewer 11systems and sewer overflow devices (SODs) on the river discharge time series.

12

133.1. Indicators derived from the literature

14The objective of this analysis is to describe and characterize the discharges time series at a 15given location. Whenever possible the indicators will be adapted/normalized so that 16comparison between different catchments of various sizes and/or various land use become 17possible.

18Hydrological regime:

19For the characterization of the hydrological regime (annual, monthly and daily time step), the 20variable time step stream flow data were first interpolated to a 30 (respectively 60) minutes 21time step for the Pollionnay, Upstream_SOD1, La Léchère stations (respectively Craponne 22and Taffignon stations) and then aggregated to daily, monthly and annual time step from 23which the hydrological regime was calculated (Sauquet et al., 2000, 2008). Missing daily 24values were replaced by the interannual monthly average corresponding to the month of the 25missing data.

26From the daily discharges we also derived the flow duration curves (FDCs). They are 27obtained by ordering all the daily discharge values in decreasing order. Each ordered 28discharge value Q_f is associated with an empirical frequency which is the probability of 29finding a value *Y*, larger or equal to Q_f .

30
$$\operatorname{Pr} ob(Y \ge Q_f) = \frac{r}{N}$$
 (1)

31 where *N* is the total number of observations and *r* is the rank of observation Q_{f} . 32

1In order to compare data from catchments of different sizes and identify the possible impact 2of land use on the discharge, it is convenient to normalize the discharge data before 3calculating the flow duration curves. The most obvious choice is to compute specific 4discharges by dividing all the data by the catchment area. The advantage is that this method 5provides water heights comparable to rainfall heights, which can be directly used to compute 6the water balance or runoff coefficients. However, in periurban catchments, especially 7equipped with combined sewer systems, this is not so obvious (Jankowfsky, 2011). 8Jankowfsky (2011) shows that, in such catchments, the boundary is not only dependent on 9topography, but is influenced by the sewer systems. The areas connected to the river via sewer 10overflow devices are only contributing to the streamflow when the sewer systems overflows. 11In addition, only part of this runoff drains into the river, because part of it continues to reach 12the Waste Water Treatement Plant (WWTP). For the Chaudanne at La Léchère catchment, 13Jankowfsky (2011) shows that under dry conditions, the area of the catchment is 2.9 km², 14whereas it reaches 4.1 km² when all the SODs are activated. This result shows that using the 15specific discharge in periurban watersheds may lead to some problems and that this quantity, 16as well as runoff height and runoff coefficients must be used with caution.

17The second option is to normalize discharge data with the average annual discharge (e.g. 18Sauquet et al., 2008). This indicator is independent of the catchment area estimation. 19However, if the mean annual discharge changes with time due to land use modifications, this 20may bias the results. Comparison between catchments is therefore more difficult and only 21normalisation based on specific discharges will be discussed in the remaining of the paper.

22<u>Low flows</u>:

23The base flow was estimated using the Tallaksen and Van Lanen (2004) algorithm on the 24daily data. The base flow was estimated by linear interpolation between n points 25corresponding to the local minima of n periods of 5-days duration without intersection. The 26BFI (Base Flow Index), which is defined as the average of the base flow divided by the mean 27interannual discharge of the time series, was selected in this study. It is a non-dimensional 28index which allows comparison between catchments. Low flow were also characterized with 29the frequency of zero daily discharge (Richter et al., 1996).

30<u>High flows</u>:

31Variable time step discharge time series were used for the discharge- duration- frequency 32analysis (QdF). The method provides a theoretical description of floods for several durations. 33The studied variable was the maximum discharge continuously exceeded during the duration 34d, QCX_d (Figure 5a) The analysis was performed in several steps.

1 • A characteristic flood duration was estimated for each gauge. The analysis was performed using data from which the base flow (calculated as described above) had been removed. As 2 3 the base flow data were available at the daily time step, an interpolation of base flow at the variable time step was performed before removing it from the variable time step time 4 5 series. An automatic program has been developed which selects the hydrographs around major floods. A discharge threshold, determined from the POT analysis (see below) is 6 7 prescribed for the events selection and all the floods higher than this threshold are retained. 8 Each hydrograph is scaled by its maximum peak discharge Q_{max} . Therefore all the 9 hydrographs have a maximum value of 1 (and can be compared amongst catchments). From 10 all the hydrographs, we compute the median hydrograph. The characteristic flood duration 11 θ is then calculated as the duration where $Q/Q_{max} = 0.5$ in this median hydrograph (Figure 5b). An iteration with the next step may be required to properly fix the discharge threshold. 12 13 • The next step is the extraction of independent values of floods for various durations d. One value per year (annual maximum) can be extracted, but in order to increase the sample size, 14 15 the Peak Over Threshold approach was used. We tested extractions with an average of 2 and 4 floods per year in the analysis. As we are interested not only in extreme floods but 16 17 also in frequent floods and given the shortness of some of the time series, the extraction 18 with 4 floods per year was finally used and will be presented below. The corresponding 19 threshold was automatically determined so that the target average flood number per year 20 was obtained (Lang et al., 1999). The procedure also ensures independence of the selected 21 events. The analysis was performed on the hydrological years (from September 1 to August 22 31) for the instantaneous flood peak (d = 0) and for d/8, d/4, d/2., d, 3d. For the various 23 gauges, the duration d was chosen close to the characteristic flood duration θ determined in

24 the previous step. Next, the empirical probability of non-exceedance of the value x

25

 $\operatorname{Pr}ob(QCX_d \leq x) = F(x)$

26 was calculated on the sampled variable QCX_d . From this relation, it is possible to calculate the quantile x(T) of return period T, associated with the F law. 27

(2)

28Hydrograph separation:

29In this study, we used the WETSPRO tool proposed by Willems (2009) to compute base flow, 30inter flow (sub-surface flow) and quick flow. This method is an extension of the recursive 31 digital filter of Chapman (1991). It assumes that the recession curve can be adjusted with an 32 exponential model. The recession constant, k, of the exponential model corresponds to the 33time in which the flow is reduced by a factor exp(-1)=0.37 during dry weather periods. The Imethod of Willems (2009) includes a second parameter w, which corresponds to the average 2fraction of the sum of the quick and inter flow volume over the total flow volume. The 3filtering procedure is first applied to extract the base flow. It is then repeated to extract the 4interflow on the (total – base flow) time series. The values of the k and w parameters were 5determined manually by trials and errors so that the base flow fraction was close to the BFI 6determined previously. The analysis was performed for the Craponne and Taffignon data 7using daily data, and the Pollionnay, Upstream_SOD1 and La Léchère gauges, using two-8hours time step data in order to capture the inter flow.

9Statistical tests:

10The Craponne gauging station was the only station with a long enough (40 years) time series 11to perform trend/rupture statistical tests. The Mann Kendall (Mann and Whitney, 1947) and 12Pettitt (1979) tests were used for trend and rupture analyses respectively. The analysed 13variables were the mean annual discharge, BFI, components of the flow as derived from the 14hydrograph separation, the POT values (using an extraction of two and four floods per year 15on variable time step time series), following the methodology proposed by Renard et al. 16(2006) and Lang et al. (2006).

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183.2. Methods specific to the existence of sewer overflow devices and sewer networks

20As said before, given the quick response time of the urban parts and the very short 21 functioning time of SODs, the indicators proposed in the previous section are not sufficient to 22 fully evidence the impact of the urban areas on the river flow, especially for small catchments 23(a few km²). Therefore, we propose specific methods, particularly adapted to these conditions. 24 These methods rely on filtering techniques of the discharge time series. Two types of 25 applications were developed (Table 3):

26<u>Filtering of SODs discharge to derive "pseudo-natural" discharge series</u>. In order to get a 27clear signature of the SOD on the river discharge time series, the SOD must be located 28upstream and close to the discharge gauging station (10 to 100 m). The filtering method is 29applicable to variable time steps time series as it takes benefit of the fact that, in case of SOD 30overflows, the discharge will increase rapidly and the measurement points will be more 31frequent. The filtering is based on a moving average with a *constant* number of points. 32Therefore the time step of the filtering will be much shorter when the response is quicker 33(activation of the SOD) than when the response is slower. The method was applied to the La

1Lèchere and Craponne stations where a SOD is present less than 100m upstream, with a 4 2points moving average. This leads to two additional time series corresponding to the SOD 3discharge (indexed SOD in Table 3) and the river discharge if the SOD was absent (indexed 4Rural in Table 3).

5<u>Filtering of the combined sewer systems discharge (CSS) series</u>. In this case, the method 6allows the separation of the measured discharge between waste water (WW), parasite clear 7groundwater infiltration in the sewer network (GW) and the runoff component (RW). Its 8application combines times series of Combined Sewer System (CSS) discharge, rainfall data, 9SOD discharge (when a SOD is present). Contrarily to the previous case, the filtering method 10is applied to fixed time step time series (in our case 6 min time step given the size of the 11catchment and the response time) in order to process simultaneously the various time series. 12The fixed time step time series are obtained from linear interpolation of the variable time step 13times series.

14The principles underlying the method are summarized in Figure 6 and detailed below.

15 • Step 1: determination of the WW fraction. This step is conducted in two phases. Step1a: During the night, as the catchment is quite small, it is assumed that the waster water (WW) 16 17 is zero. In dry period conditions, the discharge measured in the sewer system during the 18 night can therefore be assumed to be clear parasite groundwater infiltration (GW). This GW 19 water is obtained by using a moving average tracking the daily minimum discharge (in our 20 case a moving average calculated using 240 points, corresponding to 24h of 6 min time step 21 data). Step 1b: This clear parasite water infiltration is subtracted from the CSS discharge, 22 leading to a residual time series (CSS - GW). To estimate the waste water discharge, it is 23 assumed that, during dry weather periods, the CSS-GW discharge is equal to the waste 24 discharge (WW). A weekly averaged cycle is calculated from the 6 min time step data 25 during dry weather. This weekly cycle takes into account week and week-end days that 26 have specific patterns.

27 • Step 2: The WW series is substracted from the CSS data. This leads to a time series of
runoff + infiltrated groundwater data (RW + GW). The same moving average tracking the
daily minimum discharge is applied once again to separate the RW and GW discharges.

30 • Step 3: If a SOD exists, its contribution is added to the runoff because the water which
31 overflows the SOD is also generated by runoff. This leads to the final value of the runoff
32 series (RW + SOD).

1Table 3 presents a synthesis of the various time series, derived using the two filtering methods 2and the name they will be referred to in the paper. The same indicators as those presented in 3section 3.1 were applied to the filtered time series, when relevant.

4

54. Results

6The results section contains two sub-sections. First we present the results of the observed data 7analysis, based on the methods described in section 3.1. Second we present the result of the 8filtering methods presented in section 3.2

9

104.1. Analysis of observed discharge time series

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134.1.1. Hydrological regime

14Table 4 provides the values of the mean annual discharge (and specific discharge when 15relevant) and 50% quantile of the FDC. Figure 7 shows the average monthly specific 16discharge for the 5 gauged stations for the 2005-2010 common measurement period. Table 4 17and Figure 7 show that the "rural" catchments have an average annual specific discharge 18ranging between 5.5 and 6.9 l s⁻¹ km². They also exhibit similar monthly patterns with higher 19values in spring and winter and low discharge in summer and autumn (Craponne, Pollionnay, 20Upstream SOD1). On the other hand, the Taffignon station exhibits systematically lower 21monthly specific discharge than its upstream gauging station Craponne. This is also reflected 22in the average annual specific discharge which is only 5.1 l s⁻¹ km² as compared to 6.9 l s⁻¹ 23km² for Craponne. An analysis of the annual runoff coefficient (not shown) confirms that the 24Taffignon values are systematically lower than those of Craponne and the difference in mean 25(0.26 for Craponne and 0.21 for Taffignon) is significant at the 5% level (p=0.02 for the t-26test). Figure 7 shows that the La Léchère station (when considering the most favourable case 27 with a 2.9 km² catchment area) has larger values of average monthly discharge than its 28upstream station (Upstream SOD1) in summer and autumn (May to October). It can be 29explained by a larger contribution of artificialized areas in summer (see section 4.2.3).

30Figure 8a shows the flow duration curve of the daily specific discharge for the gauged 31catchments: Taffignon, Craponne, Pollionnay, Upstream_SOD1, La_Léchère. It shows that 32the Craponne station has larger specific discharge than the downstream Taffignon station for 33almost all the range of frequencies. The lowest runoff production is therefore generalised for

1the whole range of discharges (except high flows – see section 4.1.3). The downstream 2La_Léchère station has significantly higher values than the Upstream_SOD1, which is much 3less urbanized than the La_Léchère station (Figure 4), for the frequencies larger than 40%.

54.1.2. Low flows and hydrograph separation

6Low flows are characterized using the % of zero daily discharge and the BFI (Table 4). For 7the gauged catchments, the % of zero daily discharge ranges between 0.25% (Craponne) to 831% (Upstream_SOD1). Note that the Taffignon gauge has a larger frequency of zero 9discharge (5.4%) than the upstream gauging station Craponne. This confirms the decrease in 10runoff production, already observed on the annual specific discharge and the FDC, from 11upstream to downstream. On the other hand, the % of zero daily discharge decrease from 1231.2% Upstream_SOD1 to 11.1% at the downstream La Lèchère station (see also Figure 8a). 13This increase can be attributed to the impact of urbanization as the additional area is mainly 14composed of artificialized areas (Table 4).

15

16The BFI values of the observed time series range between 0.26 and 0.54. Those values are 17quite low and are characteristic of catchments with a low storage capacity. For the catchments 18with the lowest % of artificialized areas (Upstream_SOD1 (24%), Pollionnay (11%), 19Craponne (18%)), the BFI increase with the catchment area, consistently with a decrease of 20the % of days with zero discharge (Table 4). The behaviour of the Taffignon station is, once 21again, inverse to what is expected: the BFI decreases between the upstream Craponne and 22downstream Taffignon stations. The lowest specific annual discharge could therefore be 23explained by a lower base flow.

24

25Table 5 shows the results of the discharge decomposition into base flow, inter flow, quick 26flow using the WETSPRO tool. The two small rural catchments (Pollionnay and 27Upstream_SOD1) have similar values of the *k* recession constant (5 days). The quick flow 28represents 43.5% of the total flow for the Pollionnay station and 49% for the Upstream_SOD1 29catchment. This can be explained by the difference in land use (mainly forest and crops for 30Pollionnay and crops for Upstream_SOD1), and because crop land has been shown to have a 31lower infiltration capacity than forest (Gonzalez-Sosa et al., 2010). The recession constant of 32the La_Léchère station is smaller than that of the upstream station Upstream_SOD1, and the 33% of quick flow is higher (54.4% against 49%), with a lower base flow. This shows the 34impact of the SODs and the urbanized areas which is the dominant land use in the additional

larea between the two gauges. When comparing the Craponne and Taffignon stations, we can 2see that the *k* recession constant increase between the upstream and downstream gauge (28 3and 35 days respectively). However, the downstream Taffignon gauge has a smaller base flow 4(39.3% against 49.8%), as said before, and a higher portion of quick flow (37.6% against 525.6%). This corresponds to an increase of the artificialized surfaces (from 18% of the total 6catchment area at Craponne to 31% of the total catchment area at Taffignon – see Figure 4). 7This analysis confirms the expected impact of urbanization: an increase of quick flow at the 8expense of base flow, but this effect has been quantified here.

9

104.1.3. High flows

11The characteristic flood durations appear in Table 4. Figure 9a shows the normalised median 12hydrographs for the various observed time series. We remind that the base flow has been 13removed before performing the extraction. For all the catchments, Figure 9a shows that the 14rising limb of the characteristic hydrographs is very sharp (a few hours). On the other hand, 15the catchments with the lowest artificialized area (Craponne, Pollionnay, Upstream_SOD1) 16have a quite heavy and long recession, although the characteristic duration, corresponding to $17Q/Q_{max}=0.5$ is short (between 3 and 7 hours, Table 4), except for Upstream_SOD1 where it 18reaches 10 hours. The most artificialized catchments (La Léchère and to a lesser extend 19Taffignon) present characteristic hydrographs with the sharpest rising limbs (less than 5 20hours) and a very quick recession (about 5 hours). Urbanization has therefore a significant 21impact on the hydrograph shape.

22The maximum daily discharge and their corresponding specific values are provided in Table 234. Figure 10 shows the result of the QdF analysis in terms of specific discharge, for durations 24d=0 (maximum instantaneous peak discharge) and 12h for the 5 gauged stations. For all the 25stations, the flood discharge decreases sharply between the two durations, which is consistent 26with the peaky shape of the characteristic flood hydrographs (Figure 9a). For the duration d=0 27and for a return period of less than ten years, the highest floods are encountered for the 28Pollionnay, Upstream_SOD, Taffignon and Craponne stations, in decreasing order. The most 29rural and natural catchment (Pollionnay) appears the most productive in terms of high floods. 30Contrarily to what is observed for the monthly regime and the low flow, the downstream 31Taffignon station is most productive than the upstream Craponne one for high flows. 32Urbanization may be responsible for this larger runoff for the highest floods. These features 33also hold for the d= 12 h duration. The results for the La Léchère station are shown using the 34two extreme catchments areas. For the most frequent floods, the specific discharge for d=0, is

1significantly larger than that of the Upstream_SOD1 station, whatever the choice of the 2catchment area. We can deduce that the urbanization of the downstream part of the catchment 3increase the magnitude of the most frequent floods.

4<u>4.1.4. Statistical tests for the Craponne gauging station</u>

5The results of the trend and rupture statistical tests performed on the Craponne time series 6(1970-2010) are summarized in Table 6. They show a significant decrease of the % of base 7flow and inter flow at the 5% and 1% level respectively, whereas the quick flow increase 8significantly at the 1% level. The increase in maximum annual discharge is not significant, 9nor the increase in maximum peak discharge. On the other hand, the POT analysis reveals a 10significant increase (at the 5 or 10% level) of the magnitude of floods for all the durations 11except d=0, when sampling the floods with two floods per year. If the sampling is performed 12to retain four floods per year, i.e to include less intense floods, none of the trend/rupture 13remains significant (not shown). These results show that a trend is only evidenced for the 14largest floods.

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164.2. Application of the filtering methods

174.2.1 Evaluation of the SOD discharge filtering method

18The result of the filtering of SOD discharge is illustrated in Figure 11 for the La Lèchere 19station. This figure shows the initial time series, the calculated La_Léchère_SOD2 and 20La_Léchere_Rural time series. We have also added the SOD1 time series on the graph, in 21order to assess the relevance of the method. Although the La_Léchère_SOD2 seems to react 22much more frequently than the SOD1 (the frequency of zero daily discharge are 22.9 and 87% 23respectively, see Table 4), we can assume that when SOD1 is active, SOD2 should also be 24active. This can allow the validation of the timing of the filtering method. Figure 11 shows 25that the agreement is reasonable and similar results were obtained on the whole period. The 26total volume discharged by SOD2 represents 5.3% of the total discharge at La Lèchère 27gauging station.

28For the Craponne_SOD time series, no data were available for validation. The area drained by 29the SOD is estimated to be about 10% of the total catchment surface (Table 3). The total 30volume of the SOD discharge represents 7% of the total Craponne discharge. This figure is 31reasonable and comparable to that of the La Léchère catchment. In the near future, it will be 32possible to fully validate them, as the La Léchère and Craponne SODs are gauged since 2010.

1In the remaining of the paper, we will assume that the filtering method is accurate enough so 2that the corresponding data series can be analysed similarly to the truly gauged time series.

34.2.2. Hydrological regime

4When considering the reconstructed "rural" time series for the Craponne and La Léchère time 5series, the average annual specific discharge of the rural time series is 13% lower for the 6Craponne station and 5% for the La Léchère station (Table 4).

7This is confirmed (both for the Craponne and La Léchère station) by Figure 12 (top), which 8shows the fraction of the total average monthly discharge coming from the SODs. This 9contribution of the SODs to the monthly regime ranges from 2-3 to about 15% with a clear 10annual cycle. The highest values occur in summer and autumn, when the natural discharge is 11the smaller. In this period, the contribution of SODs to the discharge is therefore important in 12a period where the river can be dry (see next section). This can increase pollution problems, 13all the more than discharge is low and dilution effects cannot occur.

144.2.3. Low flows and high flows

15The frequency of zero daily discharge for the total, rural and SOD components of the 16Craponne and La Léchère stations (Figure 12, bottom), show very different behaviours 17between the two gauges. Whereas the frequency of zero discharge in the Craponne_SOD is 18quite constant throughout the year, it presents a clear annual cycle at La_Lèchère_SOD2. 19Values are higher in summer than in winter, which shows that the SOD2 is more frequently 20activated in winter, when the soil conditions are humid, than in summer when they are drier. 21For the two gauges, the % of zero daily discharge of the "rural" time series is systematically 22lower than that of the total discharge. For the La Léchère station, the difference can reach 2320% in September and October. It means that the discharge in the river only comes from the 24SOD2 during this period.

25We also see that, when the SODs contribution are filtered, the BFI values are larger than for 26the total discharge (Table 4), showing, as expected that SODs mainly contribute to quick 27flow.

284.2.3. Low flows and high flows

29Figure 9b and 9c show the normalized hydrographs for the total, rural and SOD components 30of the Craponne and La Léchère stations respectively. As expected the SOD hydrographs are 31very peaky with characteristics flood durations of about 1h. Although the total and rural 32hydrographs are very close for the La Léchère station, the rural Craponne series shows a

1much slower recession than the total curve with a doubling of the characteristic flood duration 2for the rural component. On this gauge, the impact of the SOD seems to be much influential 3on the hydrographs shapes than for the La Léchère catchment.

4Figure 13 shows the QdF analysis for the total, rural and SOD components of both the 5Craponne and La Léchère stations. This figure shows that, for the largest floods, the rural and 6total curves are very close. It suggests that the largest floods are mainly associated with a 7large contribution of the rural area. On the other hand, the impact of SODs is significant for 8frequent floods, with return period of less than 2 years.

94.2.4. Analysis of the sewer system components

10The mean annual values of the sewer discharge components are provided in Table 4. The 11waste water accounts for 33% of the total discharge, the rainwater runoff for 38% and the 12infiltration of groundwater into the sewer for 30%. The results of the filtering of the sewer 13discharge time series is illustrated in Figure 14 at the monthly time scale for the whole 14measurement period (2001-2010). It shows that the groundwater infiltration exhibits a clear 15annual cycle. It is almost zero in summer when the soil is dry and increases in winter and 16spring when the soil moisture is higher. The monthly pattern of rainwater runoff follows that 17of the monthly rainfall. Figure 8b shows the FDC of the CSS discharge and its various 18components. The FDC of the waster water (WW) is very flat as expected from its building. 19Values of CSS RW+SOD1 below 0.001 m³ s⁻¹ can be considered as noise, which explain the 20low value of zero daily discharge (about 10% in Table 4). The average fraction of rainy days 21is about 50% and the CSS RW+SOD component of the CSS discharge is only not null when 22it rains. For the values larger than 0.001 m³ s⁻¹, Figure 8b shows that the ranges of infiltrated 23groundwater and runoff discharges are very similar. Groundwater infiltration into the network 24is therefore a major problem for the efficiency of the sewer network. It may explain the larger 25activation of SOD2 in winter when the groundwater table is high (see Figure 12, bottom). 26

275. Discussion

28*The importance of data collection*. The analysis presented in this paper relies on a nested and 29complete network of gauging stations, monitoring discharges in sub-catchments with various 30land uses but also on some infrastructures such as SODs and sewer networks. The set up in 31the Chaudanne sub-catchment is very valuable, as it allows the quantification of the impact of 32SODs and infiltration into the sewer network. However, although lots of efforts were 33dedicated to monitor the various branches, it is still impossible to close the water balance of

1the catchment (uncertainty of the partition of water between SODs and the Waste Water 2Treatment Plant, partial measurements of the SODs).

3For the type of analysis presented in the paper, the data quality and length of the series is of 4prime importance. The data availability period is very variable (between 5 and 40 years). 5Except Figure 7, all the indicators are calculated using the largest available time series. Due to 6 6 internanual climate variability, which is large in this catchment (the CV of average annual 7rainfall is 241 mm at Pollionnay for an average of 746 mm on the 1997-2009 period), this 8may induce sampling fluctuations of the indicators. We verified that the conclusions were 9similar when the indicators were calculated on the same period (2005-2010). Another 10important point is the accuracy of the discharge data. A diagnostic of the gauging stations 11showed that low flows are generally quite uncertain due to very large sections and a small 12water height (Pollionnay, Upstream SOD). This problem is being solved by adding a 13triangular device to get a higher accuracy for small discharges. The Craponne and Taffignon 14stations are considered reliable by their manager (DREAL) and therefore the decrease of 15runoff from upstream to downstream cannot be attributed to measurement problems. The La 16Lèchère station is the most accurate as it is equipped with a calibrated canal, but it may be 17overflooded for the largest discharges. This can explain the decrease in specific discharge for 18 high flows (Figure 10). Branger et al. (2011) proposed a method to assess the accuracy of 19stage-discharge relationship and provide an error bound for each discharge measurement. The 20use of this information was beyond the scope of this paper, but will be useful to better assess 21the significant changes/trends. In terms of monitoring, progress is under way as new SODs 22are now monitored in the context of the quantification of rejected water in the receiving rivers 23(Water Framework Directive). These new data offer an opportunity to better understand the 24 response of urbanized areas in periurban catchments and further validate the filtering method 25leading to the decomposition of total discharge into rural and SOD components. The 26Craponne subcatchment has a quite large area and it may be difficult to discriminate between 27the influence of the various infrastructures impacting the flow. The new discharges 28measurements in the SOD will be very valuable as they will allow a better assessment of the 29limits of applicability of the SOD filtering method.

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31*Impact of artificialization on the regime and the flow components. Impact on low flows and* 32*the importance of the sewer network.* Two results of this study are particularly striking: the 33decrease of specific discharge from upstream to downstream between the Craponne and 34Taffignon gauges, and the large fraction (30% of the total discharge) of infiltration into the

1 sewer system calculated for the Chaudanne subcatchment. This figure is high but consistent 2with independent estimation conducted using punctual measurements during the night by the 3services in charge of the network management. Recently Prigiobbe and Guilianelli (2009) 4reported an infiltration of about 50% in the old Rome sewer network and 14% in a recently 5constructed area. Berthier et al. (2004) found that the contribution of soil water infiltration to 6runoff in sewer pipes was about 15% in a small residential periurban catchment. On the same 7catchment, Rodriguez et al. (2008) simulate 11% of infiltration into the rainwater sewer 8system and 18% in the wastewater sewer system. In the Yzeron catchment, the main pipes 9network was built along the valley bottom in the seventies to collect waters from the 10surroundings. Breil et al. (2010) analysed one year of discharge monitoring in the main sewer 11collector close to the Taffignon station. They estimated the infiltration into the network to be 1227% of the annual discharge. There is therefore a consistency in the results and infiltration 13into the sewer network can be considered as responsible for the decrease of base flow and 14average monthly discharge between Craponne and Taffignon. The impact of the SODs is also 15significant on low flows. They significantly impact the frequency of zero discharge and, for 16the smallest sub-catchments where the natural river regime is seasonal, water coming from the 17SODs is the only source of water during a significant part of the year. The impact in terms of 18water quality is of course important (Lafont et al., 2006). However, in order to conform to the 19water framework directive, the management of rainwater is being reconsidered: where 20possible rain waters are diverted to new separate sewers flowing to retention/detention basins. 21This process is under way in the Chaudanne catchment and should be monitored to quantify 22its future impact on the river discharge. Another lesson of this study is the importance of 23knowing precisely the catchments areas. This task is quite simple for natural catchments but is 24much more complicated for periurban catchments (Jankowfsky, 2011). However, such 25determination is essential to compare various catchments based on specific discharge. A 26normalisation with the mean annual discharge was attempted, but the interpretation of the 27results was not found relevant. This question of catchment area is therefore a major challenge 28 for data interpretation and comparison in periurban catchments.

29*Impact on high flows*. The results obtained in this paper confirm that for the highest floods, 30the impact of urbanization (via the SODs) is limited. In this case, the rural part of the 31catchment is the major contributor to floods, which can be explained by saturation of the soil 32due to long rainfall events. It may be counter-intuitive to see that the most productive 33catchment for high flows is the less urbanized one (Pollionnay). However, other studies 34(Sarrazin, 2012) have shown that the whole catchment is saturated during the largest floods

1(corresponding to long lasting events of at least 70-80 mm). The whole catchment area is 2therefore contributing to the flow, whereas a part of the rainwater is diverted towards the 3WWTP in the artificialized catchments, even if all the SODs are activated. Our results also 4confirm that SODs mainly impact frequent floods, as already shown by Radojevic et al. 5(2010) using model simulations. Urbanization and catchment artificialization have also an 6 impact on the hydrographs dynamics as shown by Figure 9. The most urbanized catchments 7have more peaky flood hydrographs. In order to refine this type of analysis, approaches such 8as those proposed by Archer (2007), Archer et al. (2010) should be investigated in the future, 9as they allow a full exploration of the whole discharge series, by analysing the frequency and 10duration of exceedance of various threshold discharges. In the Yzeron catchment, it has been 11shown that these variables were very relevant to quantify the impact of SODs on the incision 12and bank erosion risk (Grosprêtre, 2011). In the case of a upstream-downstream gradient of 13urbanization, like in the Yzeron basin, urban generated floods are flowing before the rural 14floods, avoiding peak flood accumulation. The effect of urbanization could be more sensitive 15and may be dramatic if impervious areas extension takes place upstream, increasing the 16probability of concomitancy between urban and rural floods (Ostrowski, 2000; Ostrowki et 17Bras, 2000.). Such an explanation may be invoked to explain the largest specific discharge of 18the downstream Taffingon station, as compared to its upstream Craponne one for high flows. 19Indeed, the Yzeron rivers receives a tributary flowing through a quite densely urbanized area 20before the Taffignon station.

21*Statistical tests*. Statistical tests were only applied to the longest Craponne time series. They 22highlighted a modification of the components of the flow, with a decrease of base and inter 23flow, and an increase of quick flow. The SODs in this catchment were installed between 1970 24and 1980 and therefore impact most of the available time series (1970-2010). There is also a 25significant increase of POT discharge (sampling with an average of two floods per year) for 26all the durations, except peak discharge. This can be put in parallel to the increase of 27artificialized surfaces in this catchment between 1970 (9%) and 2008 (18%), all the more than 28no significant change of maximum annual cumulated (1 to 15 days) daily rainfall can be 29evidenced. For the analysis of floods, an analysis of rainfall with a shorter time step (typically 30hourly) would be necessary but the available time series are too short up to now for such an 31analysis. In addition, those results on floods must be considered with caution as they are 32sensitive to the sampling strategy.

33*Towards a perceptual model of the Yzeron catchment behaviour*. From the data analysis 34presented before, we can propose a perceptual model of the Yzeron catchment hydrological

1 functionning. The rural part of the catchment has a high infiltration capacity (Gonzalez-Sosa 2et al., 2010) but a small storage capacity. Therefore, for the small to medium events, the 3catchment response is mainly dominated by the quick response of urban areas for the 4formation of peak discharge and a delayed rural response mainly impacting the recession 5curve. Frequent floods are increased by the catchment urbanization. For the largest events 6(mainly long duration rainfall events), the catchment response is dominated by the rural 7response due to soil saturation. The river regime is also perturbed by the sewer systems 8through a high rate of infiltration of soil water which is diverted towards the WWTP and 9contributes to a decrease of base flow in the rivers. On the other hand, SODs have an 10antagonist impact as they provide a significant contribution to the river discharge in summer, 11especially for small catchments. But their impact is smaller than that of the sewer systems and 12the whole impact is mostly a base flow decrease. However, given the actions planned to 13 improve the quality of the receiving waters (improvement of the tightness of the sewer 14systems, separation of rainwater and waste water, building of retention basin, etc), the 15 discharge monitoring must be continued to see if those actions have a real impact on the 16discharge. The indicators presented in this study can help monitoring the system.

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196. Conclusions and perspectives

20The data analysis presented in this paper provides valuable insight into the periurban Yzeron 21catchment behaviour. We were able to highlight relevant indicators of the impact of 22urbanization/artificialization on the hydrological regime of periurban rivers. The results were 23obtained thanks to the availability of a long term and rich data set, sampling various aspects 24of the periurban water cycle. These series were relevant for our analysis. However, longer 25time series and sampling of more infrastructures would be necessary to fully close the water 26balance of such catchments. We have also seen that the determination of the periurban 27catchment area is also of importance for a correct data analysis. The filtering method 28proposed to decompose the measured discharge into rural and SOD component was found to 29be efficient in highlighting the major role of SODs on the whole hydrological regime, 30including, mean monthly regime, low flows and high flows. The application of the method 31requires the location of the SOD and gauging station to be very close, and data acquired with 32a variable time step. This findings provide guidelines for the setting of proper monitoring 33networks of those periurban catchments. Finally, the role of water infiltration within the sewer

1system was highlighted. It was found to account for 30% of the total combined sewer system 2discharge. There are evidence that this infiltration has an impact on low flow and on a large 3part of the hydrological regime, by decreasing the water into the stream. This impact is 4 important to consider due to its large impact on water quality and on ecosystems.

5The indicators derived in this study are also very useful for the assessment of model results 6set up in the Yzeron catchment both at small scale (Chaudanne and Mercier catchments, 7Jankowfsky et al, 2011) and the Yzeron catchment (Branger et al., 2012).

8Future work would be required to analyse more in depth the events characteristics, such as 9done in Sheeder et al. (2002) or Furusho et al. (2012) and characterize the impact of 10artificialization on the hydrograph shape using for instance methods derived from Archer 11(2007), Archer et al. (2010).

12

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1List of figures

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3Figure 1: Map of the DTM (25m resolution) of the Yzeron catchment with location of the 4rainfall gauges and streamflow gauges. For the gauges used in the study the corresponding 5sub-catchments are delineated in yellow. The location of roads and built areas is also shown 6in grey.

7Figure 2: Maps of land use of the Yzeron catchment in (a) 1970; (b) 1990; (c) 2008. The 8original maps were obtained by manual digitalization of aerial photographs by Jacqueminet et 9al. (2011), UMR EVS, 2010. They were reclassified in 3 classes: wooded areas (broad leaved 10and coniferous forests, moors), agricultural areas (crops, pastures, orchards, gardens) and 11artificialized areas (urban parcels including gardens and parks, roads, parkings).

12

13Figure 3: Zoom of the available gauging stations on the Chaudanne river, close to the Sewer 14Overflow Device (SOD1). It shows the location of the gauging stations in the stream, the 15SOD1, the combined sewer system (CSS). The approximate location of SOD2 and La 16Léchère gauging stations is also provided.

17Figure 4: Land use of the various study catchments. Three classes are presented: wooded, 18agricultural and artificialized areas (see Figure 2 caption).

19Figure 5: (a) Illustration of the sampling strategy used in the QdF method for various duration 20*d* and d_2 . (b) Determination of the flood hydrograph shape. Light grey lines are individual 21floods normalised by their maximum value Q_{max} . The thick black line is the median of all the 22curves. θ is the flood characteristic duration corresponding to $Q/Q_{max}=0.5$ in the median 23hydrograph.

24Figure 6: Scheme illustrating the methodology retained for the decomposition of combined 25sewer system (CSS) discharge between waster water discharge (WW), clear groundwater 26infiltration (GW) and runoff (RW). SOD refers to the contribution of a SOD is present. 27

28Figure 7: Average monthly specific discharge of the Craponne, Taffignon, Pollionnay, 29Upstream_SOD1, La Léchère stations. For the La Léchère station, two graphs are provided 30corresponding to the minimum (2.9 km²) and maximum (4.1 km²) catchment area.

31

32Figure 8: (a) Flow duration curve (FDC) for the daily specific discharge of the Taffignon, 33Craponne, Pollionnay, Upstream_SOD1, La_Léchère (area 2.9 km²) stations. (b) FDC of the

1sewer discharge (CSS) and its components: rainwater runoff (RW+SOD1), infiltration of 2groundwater (GW), waste water (WW).

3Figure 9: (a) Normalised median of the flood hydrographs for the Taffignon, Craponne, 4Pollionnay, Upstream_SOD1, La_Léchère, CSS_RW+SOD1 stations. (b) Normalised median 5of the flood hydrographs for the Craponne gauge and its "Rural" and "SOD" components. (c) 6Normalised median of the flood hydrographs for the La Léchère gauge and its "Rural" and 7"SOD2" components.

8Figure 10: Specific discharge from the POT analysis of the Taffignon, Craponne, Pollionnay, 9Upstream_SOD1, La_Léchère (with surface catchment 2.9 and 4.1 km2) stations for the 10duration d=0 (peak discharge) and d=12 h. The analysis was performed extracting an average 11of four flood events per year. The x-axis provides the empirical return period of each event.

12Figure 11: Result of the filtering of the SOD2 discharge from the La Léchère gauge discharge 13time series for the 01-06 July 2007 period. The discharge of the upstream SOD1 is also 14shown.

15Figure 12: Top: fraction of the total monthly discharge (%) coming from the SODs for the 16Craponne and La Léchère stations. Bottom: fraction of zero daily discharge (%) for the 17Craponne and La Léchère stations and their filtered Rural and SOD components.

18Figure 13: Discharge from the POT analysis of the Craponne (bottom) and La Léchère (top) 19stations and their rural and SOD components for the duration d=0 (peak discharge) and d=3 h. 20The analysis was performed extracting an average of four flood events per year. The x-axis 21provides the empirical return period of each event.

22Figure 14: Partition of the monthly sewer discharge volume data into the runoff component 23(grey) and the groundwater infiltration (black) for the 2001-2010 period. The monthly rainfall 24is shown at the top of the graph. The date format in the x-axis is yy-mm where 01 is 2001 for 25instance.

1Table 1: Indicators derived from discharge time series analysis found in the literature, and 2associated references. We distinguished studies based on model simulations and data analysis. 3In bold, we highlight references dealing with catchment urbanization. With a star, the study 4uses paired catchments

Analysed indicators	Model simulation	Data analysis
Hydrological regime		
Mean annual runoff, runoff coefficient	Ashagrie et al. (2006), Beighley et al. (2003), Praskievicz and Chang (2009)	*Arrigoni et al. (2010), Bullock (1992), Wang et al. (2007)
Seasonal runoff	Praskievicz and Chang (2009)	Richter et al. (1996)
Quantiles Flow duration curve	Gustard and Wesselink (1993)	*Arrigoni et al. (2010) *Bathurst et al. (2011), Wang et al. (2007)
Water balance components	Claessens et al. (2006), Elfert and Bormann (2010)	u. (2007)
High flows		
Value and/or date of annual max peak discharge or max annual discharge other several durations	Ashagrie et al. (2006), Beighley et al. (2003), Burns et al. (2005), Praskievicz and Chang (2009)	*Arrigoni et al. (2010), *Bathurst et al. (2011), Bullock (1992),Wang et al. (2007), Richter et al (1996)
Peak over threshold (POT)	Vazquez et al. (2008), Willems (2009), Vazquez and Feven (2010)	Tong (1990),
QdF (discharge-duration-frequency)	Sauquet and Leblois (2001), Radojevic et al. (2010)	
Low flows		
Value and/or date of annual min discharge (or minimum over a period of several days)	Ashagrie et al. (2006), Burns et al. (2005), Praskievicz and Chang (2009)	*Arrigoni et al. (2010), Richter et al. (1996)
POT for low flow	Vazquez et al. (2008), Willems (2009), Vazquez and Eavan (2010)	
Base Flow Index (BFI)	1 Cycli (2010)	Bullock (1992), *Dow (2007), Wang et al. (2007)
Low flow frequency, Q _{95%}	Gustard and Wesselink (1993)	Bullock (1992)
Indicators based on hydrographs analyses		
Indicators derived from events analysis: runoff coefficient, recession curves	Chormanski et al. (2008), Meierdiercks et al. (2010), Ott and Uhlenbrook (2004)	*Buytaert et al. (2004)
Flow components based on flow separation	Vazquez et al. (2008), Willems (2009), Vazquez and Feven (2010)	
Number and/or duration of discharge exceeding thresholds		*Archer (2007), Richter et al. (1996)
Number and/or duration of rising and falling rate of discharge exceeding thresholds		Archer et al. (2010), Richter et al. (1996), Tetzlaff et al. (2005)
Results of statistical analyses		
Statistical comparison of sub-periods Trend detection Regression with landscape characteristics	Radojevic et al. (2010) Claessens et al. (2006)	Tong (1990), Wang et al. (2007) Kliment and Matoušková (2009) *Arrigoni et al. (2010), *Dow(2007), Tetzlaff et al. (2005), Wang et al. (2007)

River	Name of the gauging	Catchment area	Data availability
	station	(km ²)	5
Yzeron	Taffignon	129.00	1988/09/16 – present
Yzeron	Craponne	48.00	1969/10/27 – present
Mercier	Pollionnay	6.77	1997/01/14 – present
Chaudanne	Upstream SOD1	2.19	1997/06/21 - 2001/07/24;
			2005/01/01 - present
Chaudanne	La_Léchère	2.9 to 4.1(*)	2005/01/01 - present
Chaudanne	SOD1	≈0.16	2001/06/26 - present
Chaudanne	CSS	≈0.16	2000/11/15 - present

1Table 2: Characteristics of the gauged sub-catchments

2(*) The La Léchère area is minimum (2.9 km²) under dry conditions, and can reach 4.1 km² 3when all the SODs are activated (Jankowfsky, 2011)

1	Table	3:	Time	series	deriv	ed by	the	filtering	methods

2	2	C		
River	Name of the time series	Discharge component	Method of derivation	Catchment area (km ²)
Yzeron	Craponne_SOD	Upstream SOD contribution	4 points moving average on the variable time step series	≈4.5
Yzeron	Craponne_Rural	"Rural" discharge without the upstream SOD	4 points moving average on the variable time step series	48
Chaudanne	La_Léchère_SOD2	Upstream SOD2 contribution	4 points moving average on the variable time step series	≈1.1
Chaudanne	La_Léchère_Rural	"Rural" discharge without the upstream SOD2	4 points moving average on the variable time step series	2.9
Chaudanne	CSS_RW+SOD1	Rainwater runoff	Filtering of the 6mn Chaudanne_CSS series	≈0.16
Chaudanne	CSS_GW	Groundwater infiltration into the CSS	Filtering of the 6mn Chaudanne_CSS series	-
Chaudanne	CSS_WW	Waste water	Filtering of the 6mn Chaudanne_CSS series	-

1Table 4: Mean annual discharge Q_A (m³ s⁻¹) and corresponding specific discharge q_A (l s⁻¹ km⁻²²), 50% quantile of the flow duration curves, $Q_{50\%}$; frequency of zero daily discharge and BFI; 3maximum daily discharge Q_{max} (m³ s⁻¹), and corresponding specific discharge q_{max} (l s⁻¹ km⁻²); 4characteristic flood duration for an extraction of 4 floods per year, θ (h), and duration, d (h), 5retained for the QdF analysis. Specific discharges are not provided for the SODs and 6CSS_RW+SOD components, due to the large uncertainty on their drained areas. For the La 7Léchère station, figures in parenthesis correspond to a 4.1 km² catchment area.

Series	Q_A	q_A	$Q_{50\%}$	% of zero	BFI	Q_{max}	q_{max}	θ	d
				daily					
				discharge					
	$m^{3} s^{-1}$	1 s ⁻¹	$m^{3} s^{-1}$	-	-	$m^{3} s^{-1}$	1 s ⁻¹	h	h
		km ²					km ²		
		0	bserved ti	me series					
Taffignon	0.664	5.15	0.275	5.44	0.388	46.25	358.5	6	12
Craponne	0.332	6.92	0.169	0.25	0.486	20.94	436.2	7	12
Pollionnay	0.045	6.65	0.129	13.96	0.336	5.95	878.9	3	12
Upstream_SOD1	0.012	5.55	0.0028	31.25	0.307	0.67	305.9	10	12
La_Léchère	0.019	6.55	0.0069	11.15	0.260	1.58	544.8	1	4
		(4.63)					(385.4)		
SOD1	0.00027	-	0.000	87.00	-	0.06	-	1	1
CSS	0.00545	-	0.0039	0.24	-	0.13	-	-	-
	Deri	ved seri	es using the	he filtering r	nethods				
Craponne_Rural	0.289	6.02	0153	0.53	0.536	14.79	308.1	16	12
Craponne_SOD	0.045	-	0.003	34.31	-	6.21	-	3	1
La_Léchère_Rural	0.018	6.20	0.0056	15.96	0.269	1.50	517.2	1	4
La_Léchère_SOD2	0.001	-	0.0002	22.86	-	0.08	-	1	1
CSS_RW+SOD1	0.002	-	0.0012	9.90	-	0.13	-	1	1
CSS_GW	0.002	-	0.0015	48.33	-	0.03	-	-	-
CSS_DW	0.002	-	0.0017	0.00	-	0.002	-	-	-

1Table 5: Decomposition of the discharge into base flow, inter flow and quick flow (in % of 2the total volume) for the 5 gauged catchments. The values of the constant of the exponential 3recession model, k (days) and the w parameters are also given. 4

Gauging station	Exponential	Exponential	w (-)	%	%	%
	parameter k for	parameter k for	parameter	base	inter	quick
	the base flow	the inter flow		flow	flow	flow
	extraction (days)	extraction (days)				
Taffignon	35	12	0.25	39.3	23.1	37.6
Craponne	28	7	0.20	49.8	24.6	25.6
Pollionnay	5	3	0.28	34.6	21.9	43.5
Upstream_SOD1	5	3	0.32	30.2	20.8	49.0
La_Léchère	3.75	1.5	0.35	26.3	19.2	54.5

1Table 6: Results of statistical tests applied to the Craponne daily discharge time series. The *p*-2values of the Mann-Kendall trend tests are provided in the first line and the direction of the 3trend (+ for an increase, - for a decrease) are given in parenthesis when significant. The 4second line contains the p-value of the Pettitt test and the date of the most probable rupture is 5given in parenthesis when significant. The *p*-value are followed with (*) when the test is 6significant at the 10% level, (**) at the 5% level and (***) at the 1% level. The POT analysis 7 is applied on the QCX_d data (maximum discharge continuously exceeded during the duration 8*d*) extracted with an average of two floods per year on the variable time step discharge time 9series.

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Indicator calculated using the daily	p
discharge at Craponne from 1970-2010	Ĩ
BFI (calculated using the Tallaksen et al.,	0.12 (-)
2004 algorithm)	0.17
% base flow (from WETSPRO)	0.02** (-)
· · · · · · · · · · · · · · · · · · ·	0.13
% inter flow (from WETSPRO)	0.006*** (-)
	0.11
% quick flow (from WETSPRO)	0.001*** (+)
	0.02** (1988)
Max annual discharge	0.21
	0.63
POT <i>QCX d</i> =0 (maximum peak discharge)	0.35
	1.05
POT QCX duration d=1h30	0.04** (+)
	0.17
POT <i>QCX</i> duration <i>d</i> =3h	0.06 * (+)
	0.13
POT <i>QCX</i> duration <i>d</i> =6h	0.07 * (+)
	0.08* (1979)
POT <i>QCX</i> duration <i>d</i> =12h	0.06 * (+)
	0.21
POT <i>QCX</i> duration <i>d</i> =36h	0.07* (+)
	0.07* (1977)

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1Figure 1 2















<u>Step 1a</u>: Filtering of parasite clear ¹ groundwater (GW) from the combined sewer system (CSS) discharge



<u>Step 1b</u>: Extraction of the dry period data and building of an average weekly waste water WW cycle



Step 2: Filtering of the CSS – WW time series to derive the runoff + parasite clear groundwater discharges (RW + GW). New filtering to separate runoff RW and GW.



Step 3: If a SOD exists, total runoff is given by the RW +SOD discharge













1Figure 10



1Figure 11



Filtering of SOD2 discharge, La Lechere 01-06 July 2007











